STORMWATER MANAGEMENT REPORT

Prepared for



for

Proposed Restaurant with Outdoor Seating

for

Block 1007; Lot 25 101 South Orange Avenue Township of South Orange Village Essex County, New Jersey

Prepared by



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> Last Revised: May 2017 May 2016

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1. Introduction

The intention of this study is to analyze the stormwater drainage conditions that will occur as a result of the construction of a proposed 283 SF building addition for the proposed restaurant use located at 101 South Orange Avenue in the Township of South Orange Village, Essex County, New Jersey. The property in question is more specifically defined as a 0.455-acre parcel located on Block 1007; Lot 25, as described on the Township of South Orange Village Tax Map Sheet #10, and to be hereby referenced as the Site. The site is bordered to the south by South Orange Village Hall, PNC Bank, South Orange Avenue, commercial and residential beyond; to the east by commercial and retail uses beyond; to the north by residential, the Public Library beyond; and to the west by Scotland Road, commercial and retail uses beyond. The scope of the study includes an analysis of runoff generated by the existing and proposed conditions as well as an analysis of the resulting stormwater collection system and an underground detention basin as shown on the accompanying engineering drawings. The following items shall be addressed in this report:

- Narrative of pre and post development conditions with calculations to substantiate derived runoff coefficients and times of concentration.
- Calculations for pre and post development 2, 10, and 100-year design storm peak runoff rates from the tributary area to the proposed underground basin.
- Calculations for the proposed underground basin, including inflow hydrographs and storage volume versus depth tables.

2. Pre-Development Conditions

Under existing conditions, the site is a vacant building (formerly the municipal building). The paved area of the site currently drains to the west of the site into the county drainage system. The site contains very flat slopes ranging from 0% to 10% with a small area of steep slopes at the driveway to the north. Analysis of the existing site conditions is based on the survey prepared by Matrix New World and last revised May 11, 2016. Based on the existing contours the site breaks down into one (1) distinct drainage area which is further described below.

EDA:

Existing Drainage Area consists of 3,866 SF (19%) grassed areas, and 15,972 (81%) paved areas. The impervious area sheet flows into a series of inlets in the County Drainage System. The pervious areas sheet flows to the same entering the County Drainage System. Based on the existing topography and land cover, the minimum time of concentration of six (6) minutes was utilized.

3. Post Development Conditions

Under proposed conditions the majority of the site is to remain including the existing building. The site proposes a building addition of 283 SF, a parking lot addition and Stormwater Management System. The proposed grading for the site results one (1) overall distinct drainage area, which is further described below.

PDA

Proposed Drainage Area consists of 3,233 SF (16%) of grassed areas and 16,605 SF (84%) of paved areas. The increase of impervious coverage will run through an underground conveyance system which drains to a proposed manhole connecting to the existing 21" RCP in Scotland Road. The remainder of the site will bypass and sheet flow as existing drainage patterns do. The minimum time of concentration of six (6) minutes was utilized for this area.

Based on our review of the NRCS Essex County Survey (included in the appendix of this report), the subject parcel contains soils of the UrBOOB- Urban Land, Boonton substratum, 0 to 8 percent slopes. The URBOOB soil is classified as Type "D" soil by the "Urban Hydrology for small Watersheds Manual" published by New Jersey Department of Environmental Protection (NJDEP). For all purposes, Type "D" soil was used during design.

4. Methodology

The proposed development has been designed to comply with the Stormwater Management Regulations of the NJDEP. The proposed development is considered a "minor" development as defined by the NJDEP (over 1 acre of disturbance or ¹/₄ acre of new impervious). The site is less than one (1) acre of disturbance and proposes less than ¹/₄ acre of new impervious; therefore, NJDEP regulations are not in

effect. As such, the proposed development is not required to provide the NJDEP reductions in runoff and provide water quality measures. The increase in peak rates is de minimis.

The Stormwater management design was performed utilizing the TR55 Method as required by the NJDEP. Runoff curve numbers were determined for each drainage area and the time of concentration calculations have been prepared in accordance with the Urban Hydrology for Small Watersheds published by the NJDEP. Hydrographs were generated for each watershed under existing and proposed conditions depicting peak runoff rates and illustrating that reductions were made. These hydrographs were created utilizing HydroCAD 9.00 and are included within the appendix of this report.

Runoff "CN" values were assigned to various surfaces as follows:

Ground Cover	"CN" Value
Paved Parking	98
Grass Cover	84

5. Conclusion

The proposed stormwater management system for the development has been designed with provisions for safe and efficient control of stormwater runoff in a manner which will not adversely affect the existing drainage patterns found in the surrounding areas. The increase in peak rates is de minimis.

The following table summarizes the total peak discharge rates for existing and proposed conditions for the stormwater runoff directly offsite and runoff rate summary for the proposed underground detention basin:

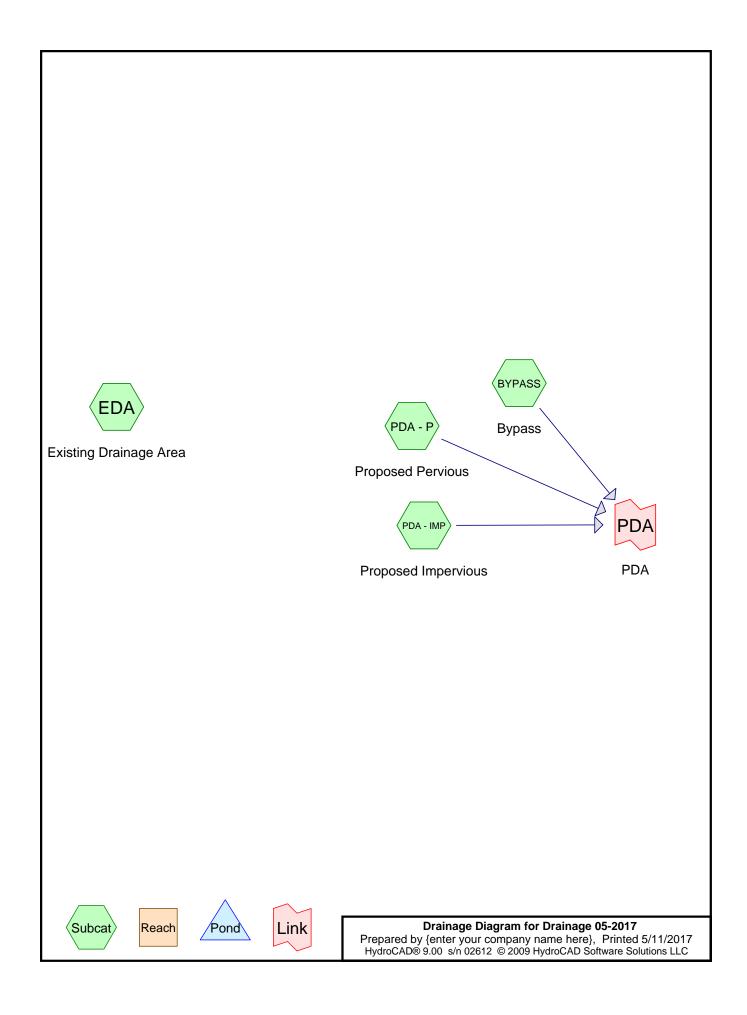
Table 1: Pre vs Post Peak Runoff Rates

(EDA VS PDA)

	(A)	(B)	
Design	Pre-Development	Post-Development	Reduction in
Storm	Runoff Rate (cfs)	Runoff Rate (cfs)	Peak Rate (cfs)
2-year	1.42	1.44	+0.02
10-year	2.23	2.25	+0.02
100-year	3.79	3.81	+0.02

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APPENDIX



Area Listing (all nodes)

Area	CN	Description	
(acres)		(subcatchment-numbers)	
0.163	84	50-75% Grass cover, Fair, HSG D (BYPASS, EDA, PDA - P)	
0.748	98	Paved parking, HSG D (BYPASS, EDA, PDA - IMP)	
0.911		TOTAL AREA	

Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Goup	Numbers
0.000	HSG A	
0.000	HSG B	
0.000	HSG C	
0.911	HSG D	BYPASS, EDA, PDA - IMP, PDA - P
0.000	Other	
0.911		TOTAL AREA

2-YEAR STORM EVENT

Drainage 05-2017 Prepared by {enter your company name HydroCAD® 9.00 s/n 02612 © 2009 HydroCAD	O Software Solutions LLC Page 4		
Runoff by SCS TR-20 r	2.00 hrs, dt=0.01 hrs, 7201 points x 9 method, UH=SCS, Split Pervious/Imperv. method - Pond routing by Dyn-Stor-Ind method		
Subcatchment BYPASS: Bypass	Runoff Area=16,616 sf 82.52% Impervious Runoff Depth=2.98" Tc=6.0 min CN=84/98 Runoff=1.20 cfs 0.095 af		
Subcatchment EDA: Existing Drainage	Runoff Area=19,838 sf 80.51% Impervious Runoff Depth=2.95" Tc=6.0 min CN=84/98 Runoff=1.42 cfs 0.112 af		
Subcatchment PDA - IMP: Proposed	Runoff Area=2,893 sf 100.00% Impervious Runoff Depth=3.21" Tc=6.0 min CN=0/98 Runoff=0.22 cfs 0.018 af		
Subcatchment PDA - P: Proposed PerviousRunoff Area=329 sf0.00% ImperviousRunoff Depth=1.89"Tc=6.0 minCN=84/0Runoff=0.02 cfs0.001 af			
Link PDA: PDA	Inflow=1.44 cfs 0.114 af Primary=1.44 cfs 0.114 af		
	c Runoff Volume = 0.225 af Average Runoff Depth = 2.97" 17.89% Pervious = 0.163 ac 82.11% Impervious = 0.748 ac		

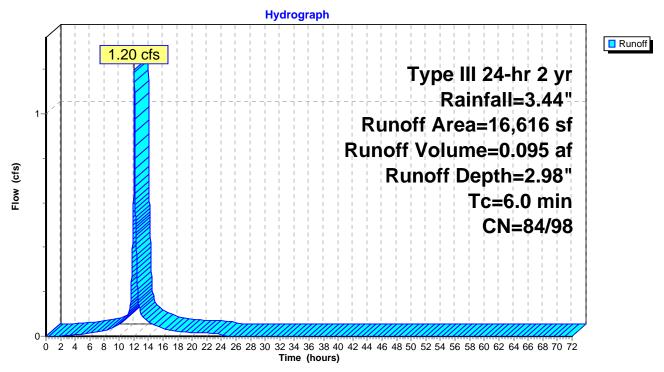
Summary for Subcatchment BYPASS: Bypass

Runoff = 1.20 cfs @ 12.08 hrs, Volume= 0.095 af, Depth= 2.98"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 2 yr Rainfall=3.44"

A	rea (sf)	CN	Description				
	13,712	98	Paved parking, HSG D				
	2,904	84	50-75% Gra	ass cover, F	Fair, HSG D		
	16,616	96	Weighted A	verage			
	2,904	84 17.48% Pervious Area					
	13,712	98 82.52% Impervious Area			ea		
Tc	Length	Slope		Capacity	Description		
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)			
6.0					Direct Entry,		
					-		

Subcatchment BYPASS: Bypass



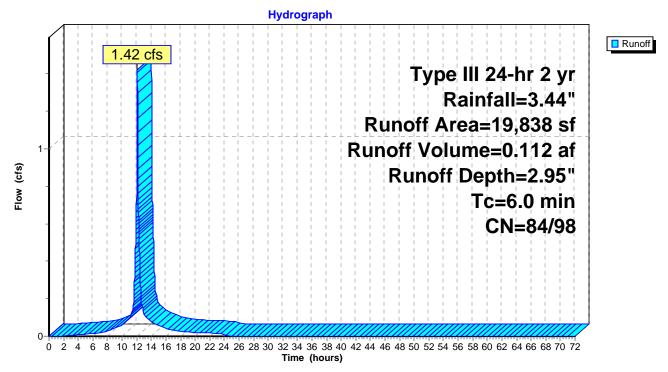
Summary for Subcatchment EDA: Existing Drainage Area

Runoff = 1.42 cfs @ 12.08 hrs, Volume= 0.112 af, Depth= 2.95"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 2 yr Rainfall=3.44"

A	rea (sf)	CN	Description			
	15,972	98	Paved park	ing, HSG D)	
	3,866	84	50-75% Gra	ass cover, F	Fair, HSG D	
	19,838	95	5 Weighted Average			
	3,866	84	19.49% Pervious Area			
	15,972	98	80.51% Impervious Area			
Tc (min)	Length (feet)	Slop (ft/ft		Capacity (cfs)	Description	
6.0					Direct Entry,	

Subcatchment EDA: Existing Drainage Area



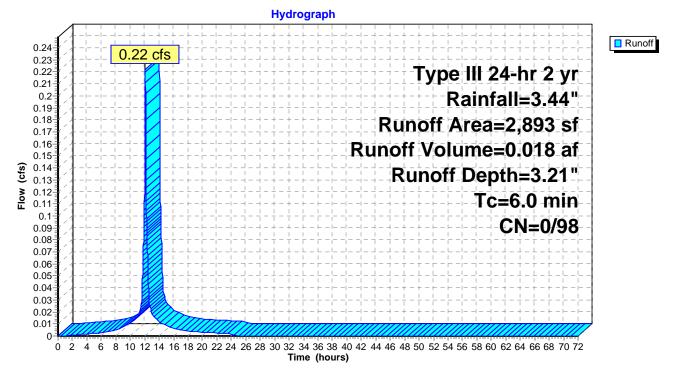
Summary for Subcatchment PDA - IMP: Proposed Impervious

Runoff = 0.22 cfs @ 12.08 hrs, Volume= 0.018 af, Depth= 3.21"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 2 yr Rainfall=3.44"

Α	rea (sf)	CN	Description				
	2,893	98	98 Paved parking, HSG D				
	2,893	98	98 100.00% Impervious Area				
Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description		
6.0					Direct Entry,		

Subcatchment PDA - IMP: Proposed Impervious



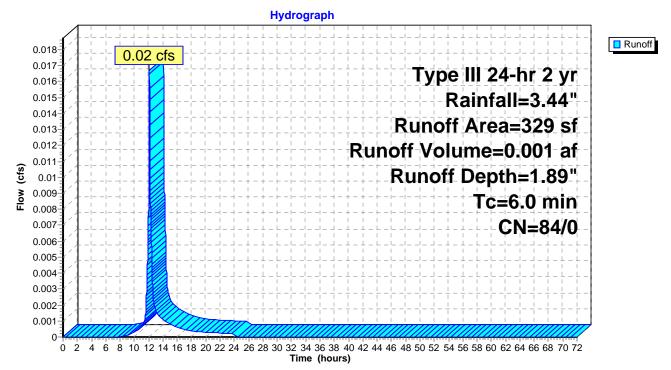
Summary for Subcatchment PDA - P: Proposed Pervious

Runoff = 0.02 cfs @ 12.09 hrs, Volume= 0.001 af, Depth= 1.89"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 2 yr Rainfall=3.44"

CN	Description			
84	84 50-75% Grass cover, Fair, HSG D			
84	84 100.00% Pervious Area			
Slope Velocity Capacity Description (ft/ft) (ft/sec) (cfs)			Description	
			Direct Entry,	
	84 84 Slope	84 50-75% Gra 84 100.00% Pe Slope Velocity	8450-75% Grass cover,84100.00% Pervious AreSlopeVelocityCapacity	

Subcatchment PDA - P: Proposed Pervious

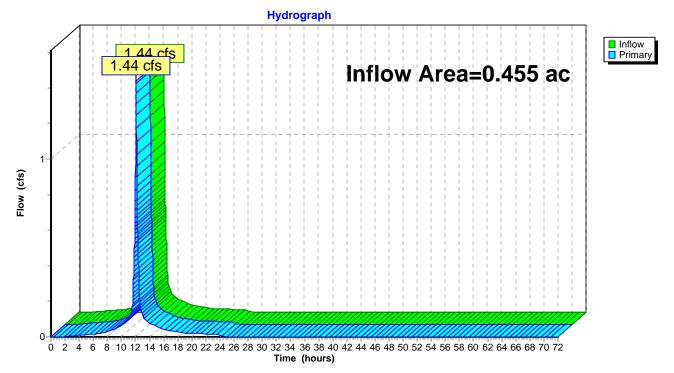


Summary for Link PDA: PDA

Inflow Area =	0.455 ac, 83.70% Impervious, Inflo	w Depth = 2.99" for 2 yr event
Inflow =	1.44 cfs @ 12.08 hrs, Volume=	0.114 af
Primary =	1.44 cfs @ 12.08 hrs, Volume=	0.114 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

Link PDA: PDA



10-YEAR STORM EVENT

Runoff by SCS TR-20 r	2.00 hrs, dt=0.01 hrs, 7201 points x 9 method, UH=SCS, Split Pervious/Imperv.
Reach routing by Dyn-Stor-Ind	method - Pond routing by Dyn-Stor-Ind method
Subcatchment BYPASS: Bypass	Runoff Area=16,616 sf 82.52% Impervious Runoff Depth=4.72" Tc=6.0 min CN=84/98 Runoff=1.88 cfs 0.150 af
Subcatchment EDA: Existing Drainage	Runoff Area=19,838 sf 80.51% Impervious Runoff Depth=4.69" Tc=6.0 min CN=84/98 Runoff=2.23 cfs 0.178 af
Subcatchment PDA - IMP: Proposed	Runoff Area=2,893 sf 100.00% Impervious Runoff Depth=4.98" Tc=6.0 min CN=0/98 Runoff=0.34 cfs 0.028 af
Subcatchment PDA - P: Proposed Perviou	s Runoff Area=329 sf 0.00% Impervious Runoff Depth=3.47" Tc=6.0 min CN=84/0 Runoff=0.03 cfs 0.002 af
Link PDA: PDA	Inflow=2.25 cfs 0.180 af Primary=2.25 cfs 0.180 af
	c Runoff Volume = 0.358 af Average Runoff Depth = 4.71" 17.89% Pervious = 0.163 ac 82.11% Impervious = 0.748 ac

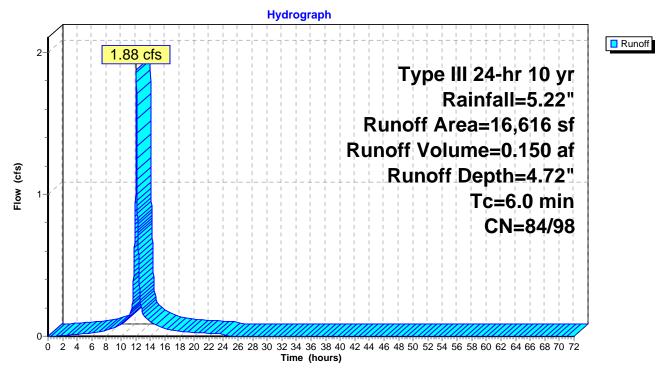
Summary for Subcatchment BYPASS: Bypass

Runoff = 1.88 cfs @ 12.08 hrs, Volume= 0.150 af, Depth= 4.72"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10 yr Rainfall=5.22"

	Area (sf)	CN	Description					
	13,712	98	Paved park	ing, HSG D)			
	2,904	84	50-75% Gra	ass cover, I	Fair, HSG D			
	16,616	96	Weighted Average					
	2,904	84	84 17.48% Pervious Area					
	13,712	98 82.52% Impervious Area						
Tc	- 3	Slope		Capacity	Description			
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)				
6.0					Direct Entry,			
					-			

Subcatchment BYPASS: Bypass



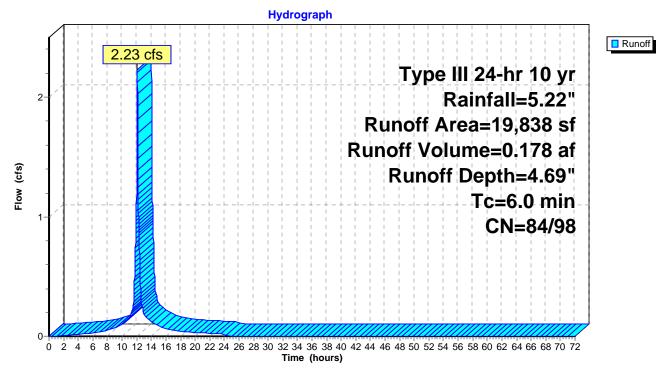
Summary for Subcatchment EDA: Existing Drainage Area

Runoff = 2.23 cfs @ 12.08 hrs, Volume= 0.178 af, Depth= 4.69"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10 yr Rainfall=5.22"

A	rea (sf)	CN	Description					
	15,972	98	Paved park	ing, HSG D)			
	3,866	84	50-75% Gra	ass cover, F	Fair, HSG D			
	19,838	95	Weighted Average					
	3,866	84	19.49% Pervious Area					
	15,972	98	80.51% Impervious Area					
Tc (min)	Length (feet)	Slop (ft/ft		Capacity (cfs)	Description			
6.0					Direct Entry,			

Subcatchment EDA: Existing Drainage Area



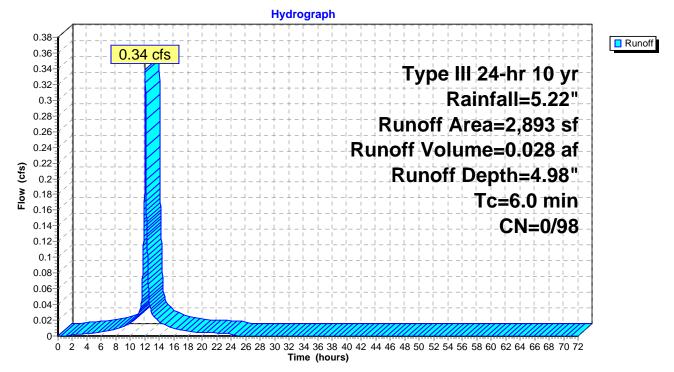
Summary for Subcatchment PDA - IMP: Proposed Impervious

Runoff = 0.34 cfs @ 12.08 hrs, Volume= 0.028 af, Depth= 4.98"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10 yr Rainfall=5.22"

Α	rea (sf)	CN	Description				
	2,893	98	98 Paved parking, HSG D				
	2,893	98	98 100.00% Impervious Area				
Tc (min)	Length (feet)	Slop (ft/ft		Capacity (cfs)	Description		
6.0					Direct Entry,		

Subcatchment PDA - IMP: Proposed Impervious



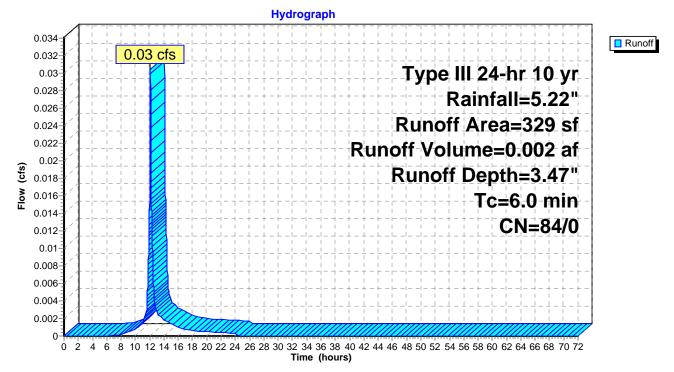
Summary for Subcatchment PDA - P: Proposed Pervious

Runoff = 0.03 cfs @ 12.09 hrs, Volume= 0.002 af, Depth= 3.47"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10 yr Rainfall=5.22"

Ar	ea (sf)	CN	Description						
	329	84	84 50-75% Grass cover, Fair, HSG D						
	329	84	84 100.00% Pervious Area						
Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description				
6.0					Direct Entry,				

Subcatchment PDA - P: Proposed Pervious

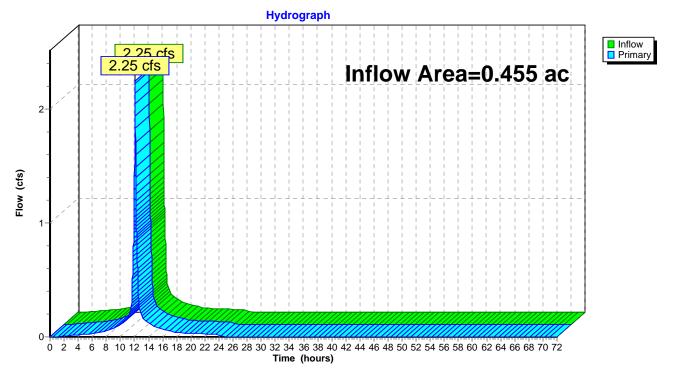


Summary for Link PDA: PDA

Inflow Area =	0.455 ac, 83.70% Impervious, Inflow	v Depth = 4.74" for 10 yr event
Inflow =	2.25 cfs @ 12.08 hrs, Volume=	0.180 af
Primary =	2.25 cfs @ 12.08 hrs, Volume=	0.180 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

Link PDA: PDA



100-YEAR STORM EVENT

Runoff by SCS TR-20 r	
Subcatchment BYPASS: Bypass	Runoff Area=16,616 sf 82.52% Impervious Runoff Depth=8.12" Tc=6.0 min CN=84/98 Runoff=3.18 cfs 0.258 af
Subcatchment EDA: Existing Drainage	Runoff Area=19,838 sf 80.51% Impervious Runoff Depth=8.09" Tc=6.0 min CN=84/98 Runoff=3.79 cfs 0.307 af
Subcatchment PDA - IMP: Proposed	Runoff Area=2,893 sf 100.00% Impervious Runoff Depth=8.42" Tc=6.0 min CN=0/98 Runoff=0.56 cfs 0.047 af
Subcatchment PDA - P: Proposed Perviou	s Runoff Area=329 sf 0.00% Impervious Runoff Depth=6.73" Tc=6.0 min CN=84/0 Runoff=0.06 cfs 0.004 af
Link PDA: PDA	Inflow=3.81 cfs 0.309 af Primary=3.81 cfs 0.309 af
	c Runoff Volume = 0.616 af Average Runoff Depth = 8.12" 17.89% Pervious = 0.163 ac 82.11% Impervious = 0.748 ac

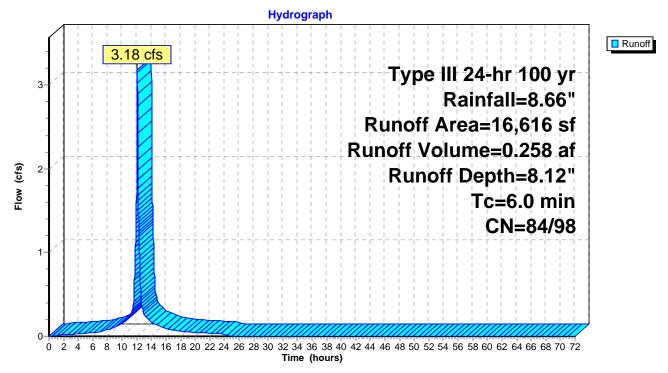
Summary for Subcatchment BYPASS: Bypass

Runoff = 3.18 cfs @ 12.08 hrs, Volume= 0.258 af, Depth= 8.12"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 100 yr Rainfall=8.66"

Α	rea (sf)	CN	Description					
	13,712	98	Paved park	ing, HSG D)			
	2,904	84	50-75% Gra	ass cover, F	Fair, HSG D			
	16,616	96	96 Weighted Average					
	2,904	84 17.48% Pervious Area						
	13,712 98 82.52% Impervious Area							
_								
Tc	Length	Slope		Capacity	Description			
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)				
6.0					Direct Entry,			

Subcatchment BYPASS: Bypass



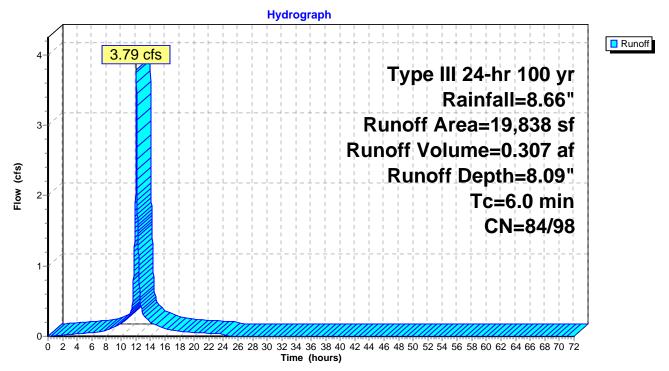
Summary for Subcatchment EDA: Existing Drainage Area

Runoff = 3.79 cfs @ 12.08 hrs, Volume= 0.307 af, Depth= 8.09"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 100 yr Rainfall=8.66"

A	rea (sf)	CN	Description					
	15,972	98	Paved park	ing, HSG D)			
	3,866	84	50-75% Gra	ass cover, F	Fair, HSG D			
	19,838	95	Weighted Average					
	3,866	84	19.49% Pervious Area					
	15,972	98	8 80.51% Impervious Area					
Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description			
6.0					Direct Entry,			

Subcatchment EDA: Existing Drainage Area



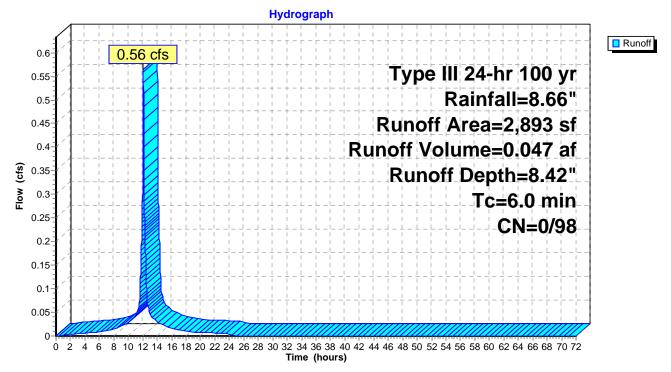
Summary for Subcatchment PDA - IMP: Proposed Impervious

Runoff = 0.56 cfs @ 12.08 hrs, Volume= 0.047 af, Depth= 8.42"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 100 yr Rainfall=8.66"

Α	rea (sf)	CN	Description				
	2,893	98	98 Paved parking, HSG D				
	2,893	98	98 100.00% Impervious Area				
Tc (min)	Length (feet)	Slop (ft/ft		Capacity (cfs)	Description		
6.0					Direct Entry,		

Subcatchment PDA - IMP: Proposed Impervious



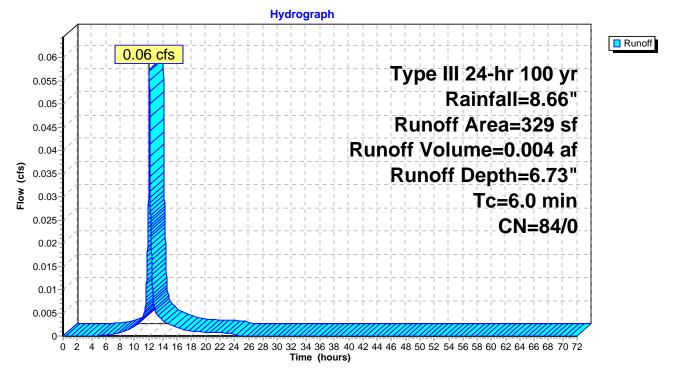
Summary for Subcatchment PDA - P: Proposed Pervious

Runoff = 0.06 cfs @ 12.09 hrs, Volume= 0.004 af, Depth= 6.73"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 100 yr Rainfall=8.66"

Are	ea (sf)	CN	Description					
	329	84 50-75% Grass cover, Fair, HSG D						
	329	84 100.00% Pervious Area						
Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)				
6.0					Direct Entry,			

Subcatchment PDA - P: Proposed Pervious

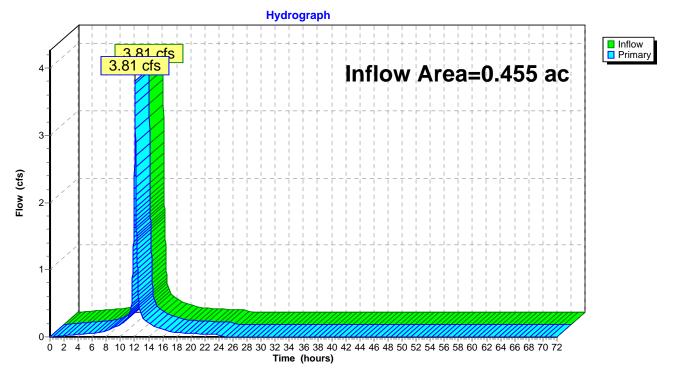


Summary for Link PDA: PDA

Inflow Area	a =	0.455 ac, 83.70% Impervious, Inflow Depth = 8.14" for 100 yr event
Inflow	=	3.81 cfs @ 12.08 hrs, Volume= 0.309 af
Primary	=	3.81 cfs @ 12.08 hrs, Volume= 0.309 af, Atten= 0%, Lag= 0.0 min

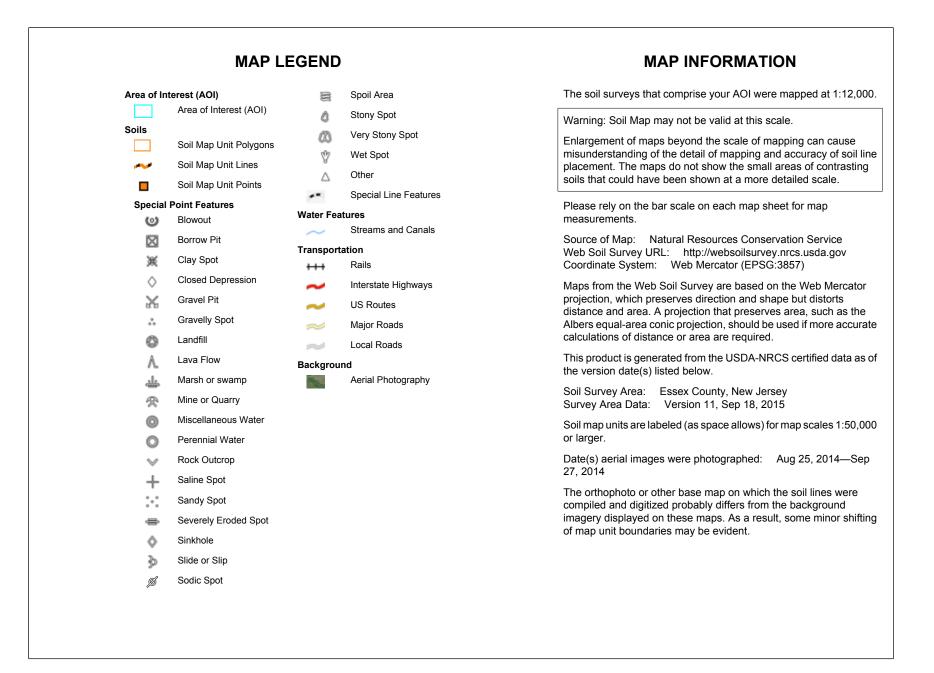
Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

Link PDA: PDA



NRCS SOIL SURVEY OF ESSEX COUNTY





Map Unit Legend

Essex County, New Jersey (NJ013)							
Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI				
URBOOB	Urban land, Boonton substratum, 0 to 8 percent slopes, red sandstone lowland	0.5	100.0%				
Totals for Area of Interest	·	0.5	100.0%				

DRAINAGE AREA MAPS

Existing Drainage Area Map Proposed Drainage Area Map

